Bridging the Micro and Macro Mechanical Behaviour of Granular Materials

Q Zhao1*, Y Chen2, G Ma3

¹The Hong Kong Polytechnic University, Hong Kong, China ²Changjiang Survey, Planning, Design and Research Co., Ltd., Wuhan, China ³Wuhan University, Wuhan, China

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ABSTRACT

Understanding the underlying physics and mechanisms responsible for the loss of stability of granular systems is crucial to the mitigation of geohazards such as landslides and earthquakes. We use a combination of in situ testing under X-ray micro-computed tomography (micro-CT) and the hybrid finite and discrete element method (FDEM) to investigate the mechanical behaviours of granular materials from the microscopic to the macroscopic scales. We conduct a miniature triaxial test on a granular column sample that is imaged with X-ray micro-CT at incremental strain steps. Then, spherical harmonic (SH) analysis is performed to characterize and reconstruct the multi-scale morphological characteristics of particles, which was used to create the digital twin of the tested sample. FDEM simulation quantitatively agrees with the overall response observed in the experiment. We find that the granular material deforms plastically through spatially localized zones of large nonaffine displacements, and the spatiotemporal evolution of these zones controls the macroscopic responses of the system. Our method sheds light on bridging length scales from microscopic scale to macroscopic granular systems.

Keywords: In Situ Test Under Micro-CT, Granular Material, Digital Twin

1 Introduction

Mechanical behaviour of granular materials in nature is related to geohazards such as landslides, soil foundation instability, and earthquakes. The granular system responds to the mechanical perturbation through the spatial rearrangement of particles and the dynamics of force transmission network (Majmudar & Behringer, 2005). Therefore, it is crucial to understand the underlying physics and mechanisms responsible for the loss of stability of granular systems.

X-ray micro computed tomography (micro-CT) is a non-destructive approach of examine material internal structures. Micro-CT have been successfully used to characterize the structural properties and microscopic scale dynamics of different materials (e.g., Andò et al., 2012; Weis & Schröter, 2017; Q. Zhao et al., 2018; Q. Zhao et al., 2020). X-ray computed tomography aided laboratory test allows for the investigation of granular materials from microscopic to macroscopic scales (e.g., Karatza et al., 2017; Afshar et al., 2018; Cheng & Wang, 2018; Kong & Fonseca et al., 2019; Chevalier et al., 2019), making it a promising tool in studying the behaviour of complex granular systems. This technology was used to measure contact forces among particles (e.g., Saadatfar et al., 2012; Andrade & Avila, 2012). However, it was only applied to granular systems with small number of particles. Tracking the trajectory of large number of moving particles and evaluating their interactions remains a challenging task, especially for the granular systems that experience large deformation.

The particle based numerical simulation methods provide a different perspective to examine the behaviour of granular systems. Previous studies suggested that particle shape has profound influences on granular dynamic behaviour, which promoted the development of advanced modelling techniques (Alshibli et al., 2016; Murphy et al., 2019), such as the discrete element method (DEM)



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(e.g., Wu et al., 2020), the hybrid finite and discrete element method (FDEM) (Ma et al., 2014; Ma et al., 2016; Chen et al., 2021), and the level set DEM (LS-DEM) method (Kawamoto et al., 2016; Kawamoto et al., 2018).

In this study, we investigate the microscopic and macroscopic behaviour of granular materials by combining in situ testing under X-ray micro-CT and FDEM modelling. With this novel combination of technologies, we achieved a quantitative description of the macroscopic responses and microscopic dynamics of the granular system during the whole deformation process.

2 Material and Methods

2.1 In Situ Testing under X-ray Micro-CT

The experiment was carried out using the in situ testing apparatus under micro-CT (ERD μ) developed by Q. Zhao et al. (2017) (Figure 1). The apparatus consists of a control panel and an X-ray transparent loading vessel (Figure 1a). Data acquisition equipment and drivers of the step motors are mounted on the control panel. During an in situ testing, the whole system is placed inside the cabinet of the micro-CT (Figure 1 b&c). This apparatus can conduct unconfined (uniaxial) compression tests, confined compression tests, and rotary shear tests.



Figure 1. The in-situ testing apparatus under X-ray micro-CT. (a) the control panel and the X-ray transparent loading vessel, (b) the X-ray micro-CT machine, and (c) the testing apparatus seated inside the micro-CT during the test.

2.2 Sample preparation and Testing Procedure

The sample is made of Ottawa sand and has an initial diameter of approximately 12 mm and a height of 25 mm. It is prepared using the dry pluviation method and subjected to vibration and tapping until a dense packing is achieved. The sample is confined by a 0.3 mm thick flexible latex membrane, providing the flexible boundary condition while allowing for the application of confining pressure. Then, the sample is placed inside the ERDµ apparatus that is mounted on the rotation table inside the X-ray micro-CT (Figure 2a). During the experiment, the specimen was first isotropically compressed

to the confining pressure (σ_3) of 300 kPa and then the axial stress (σ_1) is increased by the downward movement of the top loading platen at a constant strain rate of 0.1%/min (Figure 2b).

Three dimensional (3D) micro-CT scans of the sample were acquired at 15 strain levels. The first scan was conducted after the application of the confining pressure. During each 3D scan, 1080 two dimensional (2D) radiographic projections with a pixel size of 27.5 μ m were acquired, while the ERD μ apparatus was rotated 360° around the vertical axis. The loading platen was kept stationary during scans and the confining pressure was maintained constant throughout the experiment. The imaging of the incrementally strained sample allowed us to investigate the gradual variation of the sample internal structures.



Figure 2: Schematic diagram of the experimental setup. (a) The ERDµ X-ray transparent testing vessel. (b) The sample assembly and loading conditions in the experiment test.

2.3 Image Processing

The 2D radiographic projections were reconstructed into a 3D digital volume of the sample, with the grey values of the voxels directly related to the density of the sample at the corresponding spatial coordinates (Figure 3 a-b). Then, the digital volume was divided into two phases: air and solid, based on global thresholding. In the binarized image, contacting grains are connected due to the partial volume effect, and a mark-controlled watershed segmentation was used to separate the grains. This watershed algorithm uses the image morphology to identify the grains and separate them from each other. Finally, the separated grains are labelled for further tracking and analysis (Figure 3c). Contacts between neighbouring particles can be identified by subtracting the watershed segmented data from the binarized data.



Figure 3: Image processing procedure. (a) Raw radiographic projection images, (b) horizontal image stack after reconstruction, and (c) image thresholding, watershed segmentation, and labelling.

Particle morphological features such as the particle volume, surface area, and principal axis lengths of the particle are used in particle matching and tracking algorithms. These morphological quantities

are sensitive to the X-ray CT imaging quality. One promising approach of improving the matching accuracy is to use more morphological features ranging from particulate size to surface texture. Spherical harmonic (SH) analysis mathematically describes the general conformation of the particle shape at different scales. SH analysis is capable of characterizing and quantifying the multiscale morphological features of irregularly shaped particles (Garboczi, 2002; Zhou et al., 2015; Wei et al., 2018). The SH invariants, independent of particle translation and rotation, are adopted in evaluating the similarities of particles to track particles across multiple loading steps (Kazhdan et al., 2003; B. Zhao et al., 2017; B. Zhao et al, 2018).

We use SH function to represent the surfaces of particles extracted from image data:

$$r(\theta,\varphi) = \sum_{n=0}^{\infty} \sum_{m=-n}^{n} c_n^m Y_n^m(\theta,\varphi)$$

$$Y_n^m(\theta,\varphi) = \sqrt{\frac{(2n+1)(n-m)!}{4\pi(n+m)!}} P_n^m(\cos\theta) e^{im\varphi}$$
(2)

where $r(\theta, \phi) = \sqrt{\sum_{x,y,z} (\kappa - \kappa_0)^2}$ is the polar radius from particle centroid $(x_0, y_0, z_0), \theta \in [0, \pi]$ and

 $\varphi \in [0, 2\pi]$ are the spherical coordinates, and c_n^m is the corresponding SH coefficient. Taking $r(\theta, \varphi)$ as the input on the left side of Eq. (1), a linear equation system with $(n+1)^2$ unknowns can be obtained to determine coefficients $c_n^m \cdot P_n^m(\cos\theta)$ is the Legendre function of degree n and order m, expressed by Rodrigues's formula:

$$P_n^m(x) = (1-x^2)^{|m|/2} \cdot \frac{d^{|m|}}{dx^{|m|}} \left[\frac{1}{2^n n!} \cdot \frac{d^n}{dx^n} (x^2 - 1)^n \right]$$
(3)

We define a set of SH frequencies as $R_n(\theta, \varphi) = \sum_{m=-n}^n c_n^m Y_n^m(\theta, \varphi)$, which exhibit rotational invariant

properties independent of particle translation and rotation (Kazhdan et al., 2003), and the modulus of the SH frequency, that is, the SH rotation-invariant is calculated as:

$$\left\|R_{n}\left(\theta,\varphi\right)\right\| = \sqrt{\int_{0}^{2\pi} \int_{0}^{\pi} R_{n}\left(\theta,\varphi\right)^{2} d\theta d\varphi} = \sqrt{\sum_{m=-n}^{n} \left\|c_{n}^{m}\right\|^{2}}$$
(4)

The SH coefficients describe the general conformation of the particle shape at different scales. While a high SH degree can represent the shape of the grains with fine details, it was found that an SH degree of 15 is sufficient to describe the grains in this study. For natural sand, there are no two particulates with completely identical morphological features; therefore, the unique set of SH rotation-invariants can be used as the geometrical DNA for matching particles.





Lastly, we can identify the matching particles between adjacent loading states by evaluating the minimum L^2 -norm difference between the corresponding vectors \mathbf{R}_n (B. Zhao et al., 2018), measured by

$$\left\|R_{n}^{k}(\theta,\varphi) - R_{n}^{k+1}(\theta,\varphi)\right\| = \sqrt{\sum_{n=0}^{\infty} \left(\left\|R_{n}^{k}\right\| - \left\|R_{n}^{k+1}\right\|\right)^{2}} \quad (k = 1, 2, K, N_{s})$$
(5)

where k is the loading states and N_s is the total number of micro-CT scans. With this particle characterization and matching method, we successfully tracked the movement of almost all the particles and created a library of the multiscale morphological information of the granular material tested in the experiment (Figure 4).

2.4 Creation of the Digital Twin

We built a digital twin of the laboratory sample using the FDEM method. A typical FDEM model utilizes the cohesive crack model to achieve simulation of the fracturing process, however, in this study, we did not observe grain breakage from the experimental results, which is supported by the particle size distribution (PSD) of the grains from the first and last micro-CT scan image data (Figure 5). Thus, the FDEM model we implement considers only the interaction of grains and no grain breakage is simulated.

The digital twin of the tested sample is configured to statistically resemble the laboratory sample, in terms of, grain numbers, grain size distribution, and grain morphological features. Each grain was meshed into many second-order tetrahedral finite elements (Figure 6). In addition, a typical FDEM model involving fracturing requires an iterative calibration process (Tatone & Grasselli, 2015); however, in this study, no calibration was conducted, and the basic material parameters are assigned to the grains (Table 1).



Figure 5: Particle size distribution (PSD) of the laboratory sample at the first and last micro-CT scan data.



Figure 6: Creation of the digital twin of the laboratory sample.

| Parameter (unit) | Value |
|---|-----------------------|
| Density (kg/m ³) | 2650 |
| Young's modulus (GPa) | 92.1 |
| Poisson's ratio (-) | 0.118 |
| Friction coefficient (-) | 0.5 |
| Normal and tangential penalty (N/m ³) | 92.1×10 ¹¹ |
| Damping factor (0.03) | 0.03 |

Table 1: Material parameters used in the FDEM simulation (from Chen et al., 2021).

3 Results and Discussion

3.1 Macroscopic Stress-strain Behaviour

The FDEM model, without calibration against the experimental results, reasonably reproduces the macroscopic behaviours of the laboratory sample including the stress-strain and dilation behaviours (Figure 7). The simulation shows an earlier peak stress ratio σ_1/σ_3 than the experiment, which may be related to the micro-CT scans conducted before the peak.

The overall consistency suggests that the critical information needed in the modelling of granular material are particle shape and packing properties, which is confirmed by other researchers (Kawamoto et al., 2018). Note that, the drop in stress ratio during each scan is due to the stress relaxation caused by the loading pause during each X-ray imaging.

We choose six strain intervals (I-VI) for further analyse: 0.00% - 0.99%, 0.99% - 5.03%, 5.03% - 8.06%, 8.06% - 11.06%, 11.06% - 14.06%, and 14.06% - 18.06%, which are divided by five strain states marked A-E in Figure 7.



Figure 7: Stress-strain and dilation curves of the experimental and numerical simulated results. Strain point A-E divides the loading process into six incremental strain steps (I-VI) that are further evaluated.

3.2 Microscopic Particle Dynamics

We identified 15961 individual particles in the tested sample. We further compare the incremental particle displacements at six strain states to examine the particle kinematics obtained from the experiment and the FDEM simulation (Figure 8). The FDEM simulated particle displacement field is in good agreement with that obtained in the experiment. The magnitude of particle displacement increases with the shearing process and bulging are shown in all the steps. Two cone-shaped dead zones are developed at the top and bottom of the sample at the post-peak softening stages. While in the middle of the sample, a zone of intensive shearing localizes into an X-shaped shear band.



Figure 8: Particle displacements at incremental strain steps for (a) experimental test and (b) FDEM simulation.

At the microscopic scale, the grains rearrange themselves in response to the shear deformation, and the varying local structures of particles creates highly heterogeneous particle motion. As a result, granular materials deform in a nonaffine form (Ma et al, 2018), that is, non-uniform rotation, deformation and rearrangement of particles. The micro-CT image data and FDEM simulation allow us to quantify the nonaffine motion of particle using the local minimum nonaffine displacement D_{min}^2 that measures the mean square deviation of the particle's position from the best-fit affine transformation of its neighbourhood over the strain interval $\delta \varepsilon$ (Falk & Langer, 1998; Ding et al., 2014; Cubuk et al., 2017):

$$D_{\min}^{2}\left(\varepsilon,\varepsilon+\delta\varepsilon\right) = \frac{1}{N_{i}} \sum_{k}^{N_{i}} \left[\boldsymbol{\mu}_{ki}(\varepsilon+\delta\varepsilon) - \boldsymbol{\Lambda}_{i}(\varepsilon)\boldsymbol{\mu}_{ki}(\varepsilon)\right]^{2}$$
(6)

where $\Lambda_i(\varepsilon)$ is the best-fit affine deformation tensor of particle *i* extracted by minimizing the $D_{\min}^2(\varepsilon, \varepsilon + \delta \varepsilon)$ (Chikkadi & Schall, 2012; Guo & Zhao, 2014):

$$\Lambda_i = \mathbf{X} \cdot \mathbf{Y}^{-1} \tag{7}$$

$$\mathbf{X} = \sum_{k}^{N_{f}} \boldsymbol{\mu}_{ki}(\varepsilon + \delta \varepsilon) \otimes \boldsymbol{\mu}_{ki}(\varepsilon)$$
(8)

$$\mathbf{Y} = \sum_{k}^{N_i} \boldsymbol{\mu}_{ki}(\varepsilon) \otimes \boldsymbol{\mu}_{ki}(\varepsilon)$$
(9)

where $\mu_{ki}(\varepsilon)$ is the displacement vector between the reference particle *i* and its neighbour *k* at strain state ε . Note that the nonaffinity measure depends on the size of the neighbourhood. Here, we take the first minimum of the pair-correlation function as the cutoff distance (Chikkadi & Schall, 2012; Chen et al., 2021). Our results showed that probability distribution functions of the local minimum nonaffine displacement D_{\min}^2 exhibits a power-law decay, which implies that particles with large nonaffine displacements are collectively organized, and particles with similar nonaffine displacements tend to form compact clusters (Kou et al., 2018; Chen et al., 2021).

The local strain tensor is determined based on the affine tensor Λ using: $\varepsilon^L = -(\Lambda + \Lambda^T)/2$, and the local deviatoric strain for each particle can be calculated as $\varepsilon^L_q = \sqrt{\frac{2}{3}} \varepsilon^L_{dev} : \varepsilon^L_{dev}$. The local deviatoric strain

 ε_a^L are evaluated at strain intervals I-V (Figure 9).

Similar spatial distribution and temporal evolution of the granular system are observed in the experimental and numerical results. At 1.0% axial strain, a few localized plastic zones appeared randomly in the elastically deformed surroundings. After the onset of yielding, particles rearrange themselves to accommodate the increased strain, and many activated plastic zones appeared. These

zones are characterized by close-packed particles with high local strain magnitude. Eventually, those plastic zones coalescence into a conjugate X-shaped shear band spanning the granular system. Once the shear band is formed, the subsequent plastic activity is essentially dominated by the evolution of the shear band through thickening and sliding.

Although the evolution patterns of the granular systems observed in experiment and simulation show great similarities, the thickness and spanning region of the shear band are slightly different. This may be related to the fact that the FDEM simulation lacks the consideration of multiscale contacting behaviour of particles. The FDEM contact model assumes the contact point is fully sticking before reaching the Column frictional resistance. For real particles with multiscale morphological characteristics may experience relative motion even under tiny perturbation and affect the local rearrangement of particles (Kou et al., 2017). In addition, a constant sliding friction coefficient is applied to the FDEM model, which ignores the variable friction conditions in actual particle surfaces.



Figure 9: The spatial distribution of local deviatoric strain at strain states I-V for both (a) experiment and (b) numerical simulation. The yellow and red indicate particles subjected to large local deviatoric strain, and particles with small are coloured black and are transparent.

In this study, the grain size is relatively large at the millimetre scale, and with the high-resolution micro-CT scan, almost all the grains were captured with high image quality. In some cases, for example, when clay samples are examined, the grain size may be at the micrometre scale that the micro-CT imaging may not be able to resolve each individual grain. In those cases, segmentation-based image analysis would not be applicable. Instead, a direct estimation of the spatial variation of density can be achieved through a correlation between grayvalues and density.

4 Conclusions

In this work, weinvestigate the mechanical behaviour of granular materials by combining the merits of the in-situ testing under X-ray micro-CT and FDEM numerical modelling. We perform spherical harmonic analysis to characterize and reconstruct the multiscale morphological characteristics of irregularly shaped particles, which enables accurate matching of particles even at the large strain intervals. Relying on the detailed spherical harmonic reconstruction, we establish high fidelity numerical model with the consistent particle morphology and disordered structure. The FDEM simulation results quantitatively agree with the overall response recorded in the experiment test. The particle scale dynamics including the nonaffine particle displacements and particle clustering behaviour show a remarkable quantitative agreement between simulation and experiment. Our results demonstrated that the spatiotemporal evolution of localized microscopic scale plastic deformation controls the macroscopic responses of the granular system. The proposed tool in this study sheds light

on bridging the microscopic and macroscopic mechanical behaviour of granular materials by 3D visualization and quantitative analysis. This providing us with a better understanding of the physical mechanisms behind the failure of granular systems, such as failure of soil slopes and weakening of gouge filled fault zones.

5. Declarations

5.1 Acknowledgements

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